

1 Design 3D printing cementitious materials via Fuller Thompson theory
2 and Marson-Percy model

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8 **Abstract:**

9 Cementitious materials for 3D printing have special requirements for rheological properties,
10 which are significantly affected by many factors, including sand gradation and packing fraction.
11 Fuller Thompson theory and Marson-Percy model are classic approaches for sand gradation and
12 packing fraction optimization, respectively. This paper attempts to apply Fuller Thompson
13 theory and Marson-Percy model in designing cementitious materials for 3D Cementitious
14 Materials Printing (3DCMP). Various gradation methods adopted in this study were Fuller
15 Thompson gradation (mixture A), uniform-gradations (mixture B and C), gap-gradations
16 (mixture D and E). Besides these mixtures with special gradation approaches, one mixture using
17 natural river sand (mixture F) was prepared as well. Rheological properties were characterized
18 by static/dynamic yield stress and plastic viscosity in Bingham Plastic model. Buildability was
19 examined by printing a column with 10 cm inner diameter via a gantry printer. Rheological test
20 results indicate that mixture A designed by continuous gradation possesses the highest
21 static/dynamic yield stress and lowest plastic viscosity. During printing test for buildability,
22 mixture A can easily reach up to 40 layers without notable deformation, while all other mixtures
23 deformed noticeably and fell down before the 35th layer. Finally, a large-scale printing was

24 carried out with mixture A and a structure with the height of 80 cm was printed successfully
25 without notable deformation. Density, compressive strength and flexural strength of printed
26 filaments were also characterized. Mechanical performance test results illustrate mixture A has
27 the highest density and appropriate compressive strength, and a relative high flexural strength at
28 different curing ages. These results indicate that Fuller Thompson theory and Marson-Percy
29 model can serve as a reasonable guide for material rheology design for 3DCMP.

30 Key words: Fuller Thompson theory; Marson-Percy model; 3D cementitious materials printing;
31 Bingham Plastic model

32 1 Introduction

33 3D Printing, also referred to as Additive Manufacturing, is a technology which builds a solid part
34 via a layer-by-layer process [1]. Due to its advantages such as customized production, reduced
35 waste, and diminished lead-time of rapid prototype [2], 3D Printing has attracted much attention
36 from various fields including building and construction [3]. In the last decade, much research has
37 been conducted in the field of 3D Printing for Building and Construction, especially in the
38 development of printing systems such as Mortar Printing [3–5], FreeFab [6], Contour Crafting
39 [7–9] and Robotic printing system [10,11]. While most of the 3D Printing for Building and
40 Construction processes can be classified as 3D Cementitious Materials Printing (3DCMP), little
41 research has been done on how to design materials for 3DCMP [12–14], especially from the
42 aspect of material design methods for 3DCMP.

43 Materials used in 3DCMP need to meet certain specific rheological requirements [3,15]. The
44 most essential steps in 3DCMP are conveying mixed materials to the nozzle via a delivery
45 system and depositing materials to build the solid object in a layer-by-layer manner. In the
46 conveying step, the materials are required to have good pumpability, which indicates how easily

47 material can be conveyed; and in the deposition step, the materials are required to have good
48 buildability, which indicates how well the materials can be stacked stably. Both pumpability and
49 buildability are closely related to the rheology performance of materials, namely static/dynamic
50 yield stress and plastic viscosity. Static yield stress is the minimum shear stress required to
51 initiate the flow and dynamic yield stress is the critical shear stress below which the shear stress
52 is insufficient to maintain the flow. Plastic viscosity is the resistance of a fluid to flow when the
53 fluid is flowing. All these rheological properties are attributed to the inter-particle force [16].
54 Typically, higher static/dynamic yield stress and plastic viscosity would enhance buildability and
55 hinder pumpability [17] and thus, seeking a balance between buildability and pumpability is
56 critical in material rheology design for 3DCMP.

57 It is well known that aggregates take up 60-80% of the total volume of cementitious materials,
58 the most commonly used building and construction material worldwide[19]. Rheological
59 properties of cementitious material are highly affected by the gradation of aggregate [18–20].
60 Good aggregate gradation contributes to high density and proper rheological properties of
61 materials. Fuller Thompson theory is a classic theory for gradation design. In 1907, W.B Fuller
62 and S.E. Thompson proposed a theory for gradation design based on experimental results[21].
63 Later, Federal Highway Administration (FHWA) proposed a modified Fuller Thompson
64 equation [22]. Fuller Thompson theory has been widely used in producing high performance
65 concrete [20,23], designing sustainable concrete with minimum content of cement [24] and
66 optimizing rheology [25].

67 Rheological performance is significantly affected by the packing fraction of a system as well,
68 which is defined as the ratio of solids to the total volume. Theoretically, materials designed by
69 the continuous gradation can achieve maximum packing fraction, which describes a condition

70 where the void volume reaches minimum for a given system. According to Marson-Percy model
71 [26], the highest packing fraction of materials results in the lowest plastic viscosity.

72 To fulfill the material rheological property requirements in 3DCMP, Fuller Thompson theory
73 and Marson-Percy model were applied in materials design of 3DCMP. Six different mixtures
74 were prepared with different gradation approaches using five different sands (0.6-1.2 mm, 0.25-
75 0.6 mm, 0.15-0.25 mm, less than 0.15 mm and natural river sand), i.e. mixture A designed by
76 Fuller Thompson theory, mixture B and C designed by uniform-gradations, mixture D and E
77 designed by gap-gradations and mixture F using natural river sand without special gradation
78 design. Rheological tests were carried out to investigate the fresh performance of all mixtures,
79 and printing tests for buildability were conducted among different mixtures via a gantry printer.
80 Density and mechanical performance (compressive and flexural properties) were characterized as
81 well. It should be noted that the time and temperature effect of rheological/material properties is
82 not considered due to the limited scope of this study. Furthermore, all tests including printing are
83 conducted within 30 minutes of cement mixing and under constant lab temperature of 26°C.

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85 2 Theory

86 2.1 Built-up theory and Bingham model

87 Perrot et al. established the relation between static yield stress and buildability of 3DCMP [17].

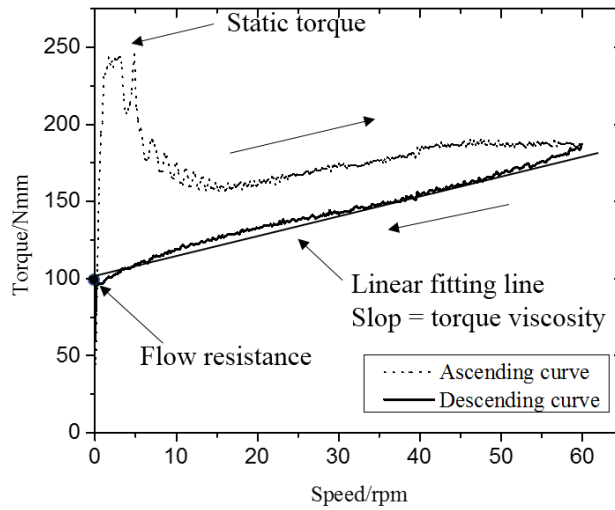
88 The simplified relation is expressed as follows:

$$89 \quad H = \frac{\alpha}{\rho g} \tau_s \quad (1)$$

90 where H (m) and α are the printed height (buildability) and the geometric factor [17] of printed
91 structure respectively; ρ (g/cm³) and g (m/s²) are the density of materials and gravitational

92 constant respectively. τ_s (Pa) is the static yield stress, which is corresponding to static torque in
 93 Fig. 1

94 Fig. 1 presents the typical rheological test result via a Schleibinger Viskomat XL rheometer with
 95 vane probe. The static yield stress can be converted from the maximum torque (also referred to
 96 as static torque) in the ascending curve. Based on linear fitting of descending curve, its slop and
 97 intersection with Y axis are defined as torque viscosity and flow resistance respectively, which
 98 can then be converted into plastic viscosity and dynamic yield stress in Bingham Plastic model.
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Fig. 1 Typical rheological test result

103 Bingham Plastic model is widely accepted to describe the rheological property of cementitious
 104 materials [27–33], which has the shear stress τ (Pa) expressed as

105
$$\tau = \tau_d + k\dot{\gamma} \quad (2)$$

106 where τ_d (Pa) and k (Pa s) are dynamic yield stress and plastic viscosity, respectively. $\dot{\gamma}$ (1/s) is
 107 the shear rate.

108 From Eq.(1), the buildability is proportional to the static yield stress. It should be noted that in
109 this study, the rheological properties are assumed to be constant during the whole printing
110 process as the time consumed for printing test (around 2 minutes) is much shorter than
111 rheological test (10 minutes). The effect of time on the rheological performance, printability and
112 buildability will be investigated in the future. It also worth noting that in this study, we focus on
113 the effect of yield stress and plastic viscosity on the printability and buildability and the
114 thixotropic behaviour and the rate of re-flocculation are out of our scope.

115 Generally, materials' pumpability is reflected by pumping pressure during pumping process,
116 which is related to Bingham Plastic parameters as well. The relationship can be described by [34]

$$117 \quad P = \left(\frac{8\tau_d}{3R} + \frac{8k}{\pi R^4} Q \right) L \quad (3)$$

118 where P (Pa) is the pumping pressure. L (m) and R (m) are the length and radius of pipe,
119 respectively. Q (m³/s) is the average flow rate. In this case, R is much less than 1 m, which
120 means that plastic viscosity (k) possesses more significant effect on the pumping pressure,
121 compared with dynamic yield stress (τ_d). Eq. (3) shows that the increases of dynamic yield stress
122 and plastic viscosity augment the pumping pressure of cementitious material for specific pipe
123 and flow conditions, which reduces the pumpability of cementitious material. Therefore, a low
124 plastic viscosity is required for cementitious materials to decrease the pumping pressure, which
125 will facilitate a safe and economic operation of pumping/printing process.

126 2.2 Fuller Thompson theory and Marson-Percy model

127 Gradation is an important attribute to produce economical cementitious materials with maximum
128 density and minimum void content. Sand gradation formula proposed via W.B. Fuller and S.E
129 Thompson is expressed as following [21]

130
$$p_i = \left(\frac{d_i}{D} \right)^{0.5} \quad (4)$$

131 where p_i is the percent pass i^{th} sieve, d_i (mm) is the opening size of i^{th} sieve, and D (mm) is the
132 maximum particle size. Later, in 1962, FHWA published a modified version for Fuller's
133 equation

134
$$p_i = \left(\frac{d_i}{D} \right)^{0.45} \quad (5)$$

135 Three types of gradation are schematically shown in Fig. 2. Applying Fuller Thompson equation
136 in sand gradation design forms a continuous gradation, in which interaction between particles
137 exists at many points of contact and realizes a maximum densified system. Gap gradation is
138 designed via missing a certain range of sand particle size in the paste, which possesses more
139 voids content and less contact points compared with the continuous grading system. Using single
140 size sand in the mixture can achieve uniform gradation mixture which has minimum contact
141 points between particles. Since contact points contribute to the friction in the mixture, increasing
142 the contact points between particles can raise the friction and, thus, resulting in increased static
143 yield stress to initiate flow and dynamic yield stress to maintain the flow.

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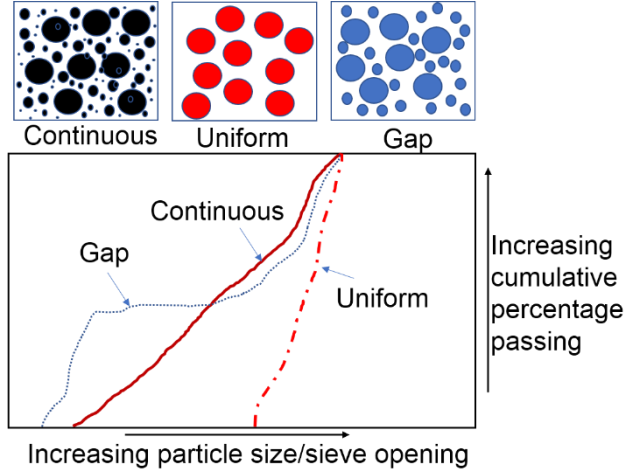


Fig. 2 Three types of sand gradation

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148 Shear viscosity μ is affected by packing fraction ϕ , maximum packing fraction ϕ_m and shear rate
149 $\dot{\gamma}$. The relationship between packing fraction, shear rate and shear viscosity is commonly
150 expressed by Marson-Percy model [26]

151
$$\mu(\phi, \dot{\gamma}) = \left(1 - \frac{\phi}{\phi_m}\right)^{-2} \mu_b(\dot{\gamma}) \quad (6)$$

152 where $u_b(\dot{\gamma})$ (Pa s) is the shear viscosity of the binder, which is the same among all samples. In
153 our study, ϕ is kept as a constant and ϕ_m is variable for mixtures with different gradation.
154 Theoretically mixture designed by Fuller Thompson theory can achieve a maximum density and
155 minimum void system, which means ϕ_m is the maximum among all mixtures, it should possess
156 the lowest shear viscosity μ .

157

158 The relation between shear viscosity and plastic viscosity of Eq.
159 **Error! Reference source not found.** can also be expressed as follows:

160
$$\mu = k + \frac{\tau_d}{\dot{\gamma}} \quad (7)$$

161 Fuller Thompson theory indicates that when the mixture designed by Eq. (4), it possesses the
162 highest dynamic yield stress (τ_d) and lowest shear viscosity (μ from Eq. (6)) among all mixtures.
163 Therefore, Eq. (7) indicates that the plastic viscosity (k) of mixture designed by Eq. (4) is the
164 smallest as well when the shear rate is the same among different mixtures. As mentioned before,
165 a low plastic viscosity is required for cementitious materials to decrease the pumping pressure,
166 which will facilitate a safe and economic operation of pumping/printing process.

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168 3 Materials, and mixture proportion

169 3.1 Materials

170 Mixtures in this study consist of Ordinary Portland Cement (OPC, ASTM type I, Grade 42.5),
171 silica fume (SF, undensified, Grade 940, Elkem company), silica sand, fly ash (FA, Class F),
172 natural river sand, water and superplasticizer (SP). Fig. 3 illustrates the particle size distribution
173 of cement, SF and FA, which were analyzed by Mastersizer 2000. Table 1 illustrates the
174 chemical composition of OPC and FA, respectively. Silica sand with four different sizes, i.e.,
175 0.6-1.2 mm, 0.25-0.6 mm, 0.15-0.25 mm, and less than 0.15 mm, were used in this study, either
176 as mono-sized ingredient or a mixture of all four.

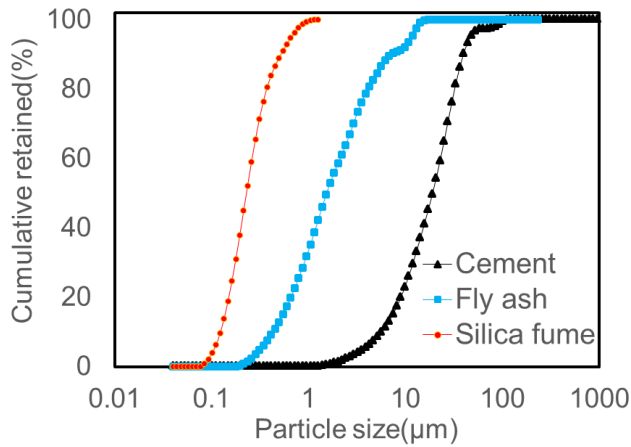


Fig. 3 Particle size distribution of OPC, FA and SF

Table 1 Chemical composition of fly ash and Ordinary Portland Cement (OPC)

Formula	Concentration / %	
	Fly ash	Cement
SiO ₂	58.59	24.27
Al ₂ O ₃	30.44	4.56
Fe ₂ O ₃	4.66	3.95
TiO ₂	2.02	0.55
K ₂ O	1.51	0.61
CaO	1.21	62.2
MgO	0.776	3.34
P ₂ O ₅	0.531	0.15
Na ₂ O	-	0.21
SO ₃	0.0914	-
ZrO ₂	0.04	-
MnO	0.0351	-
Cr ₂ O ₃	0.027	-
CuO	0.0254	-
ZnO	0.0229	-

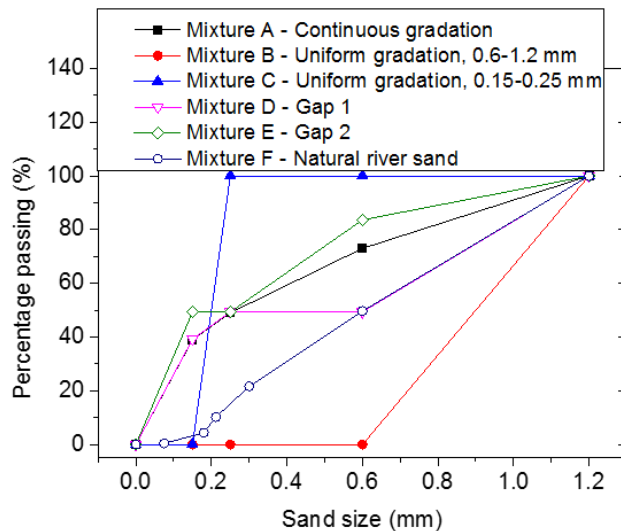
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3.2 Mixture proportion

Six different mixtures, including five mixtures with silica sands following different gradation and one mixture with natural river sand, were used in this study. The sand gradation curves are shown in Fig. 4. Sand for mixture A followed the Fuller Thompson theory to achieve a continuous gradation system. The continuous gradation mixture used in this study is 39.23% of sand size less than 0.15 mm, 10.14% of sand with 0.15-0.25 mm, 23.84% of sand with 0.25-0.6

188 mm, and 26.80% of sand with 0.6-1.2 mm. Sand for mixture B was a uniform gradation
 189 mixture with sand size ranging from 0.6 mm to 1.2 mm. Sand for mixture C was another
 190 uniform gradation mixture with sand size ranging 0.15 -0.25 mm.
 191



192
 193 *Fig. 4 Sand gradation curves for different mixtures*
 194

195 As can be seen from Fig. 4, the gap gradation 1 (mixture D) used in this study removed the
 196 sand particle size between 0.25-0.6 mm, and was composed of 39.23% of sand particle size less
 197 than 0.15 mm, 10.14% of sand particle size with 0.15-0.25 mm, and 50.63% of sand particle size
 198 with 0.6-1.2 mm. Gap gradation 2 (mixture E) removed sand particle size between 0.15-0.25 mm,
 199 and is composed of 49.37% of sand particle size less than 0.15 mm, 23.84% of sand particle size
 200 with 0.25-0.6 mm, and 26.69% of sand particle size with 0.6-1.2 mm.

201 Mixtures F used the natural river sand as the raw materials while the sand with particle size
 202 bigger than 1.2 mm was sieved out. Sieved natural river sand was then analyzed by sieving
 203 machine. The sieved natural rived sand consists of 0.86% of sand with particle size less than
 204 0.075 mm, 6.99% of sand ranging from 0.075 to 0.18 mm, 3.68% of sand ranging from 0.18 to

205 0.212 mm, 11.24% of sand ranging from 0.212 to 0.3 mm, 32.87% of sand ranging from 0.3
206 to 0.6 mm and 44.36% of sand ranging from 0.6 to 1.2 mm, as shown in Fig. 4.

207 All the mixtures mentioned above, except silica sand itself and natural river sand, followed the
208 same mixture proportion, as shown in Table 2:

209 *Table 2 Mixture proportion*

OPC	Sand	W	FA	SF	SP/(g/L)
1	0.5	0.3	1	0.1	1.3

212 Note: all ingredients content are expressed as weight proportion of cement content

213

214 4 Mixing and Testing Procedures

215 4.1 Mixing procedures

216 A hobart mixer X200L was used for mixing. Since many factors can affect the rheological
217 properties of cement slurries, such as mixing time, mixing speed and temperature [35], the
218 mixing procedures in this study were fixed to minimize the differences among batches. Firstly,
219 the powder of all solid ingredients was dry mixed for 3 minutes in stir speed; then water was
220 added, the mixing process continued for 3 minutes in stir speed followed by 2 minutes in speed I;
221 and then the SP was added, the mixing process continued for 1 minute in stir speed followed by
222 3 minutes in speed I.

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224 4.2 Rheological characterization

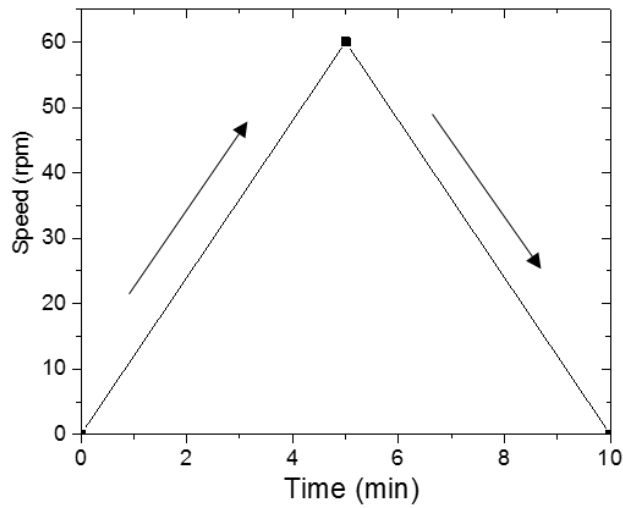
225 Rheological properties of mixed materials were evaluated via Viskomat XL and mini-slump. The
226 six-blade vane probe and cage used for rheological test to avoid slippage of cement paste. Both
227 the diameter and the height of this vane probe are 69 mm, and the gap between probe and cage,
228 and the bottoms of probe and barrel are both 40 mms. During the rheological test, the speed of
229 rheometer is increased linearly from 0 rpm to 60 rpm in 5 minutes. Afterwards, it is decreased

230 linearly to 0 rpm in another 5 minutes as shown in Fig. 5. Then the static/dynamic yield stress
 231 and plastic viscosity can be computed by Chhabra et al [34]:

$$232 \quad \Gamma = \frac{4\pi R_1^2 R_2^2 l \eta}{R_2^2 - R_1^2} \omega_2 - \frac{4\pi R_1^2 R_2^2 l \tau_0}{R_2^2 - R_1^2} \ln \frac{R_1}{R_2} \quad (8)$$

233 where Γ (N m) is the torque, ω_2 (rad/s) is the rotational speed of outer barrel, l (m) and R_1 (m)
 234 are the length and radius of the probe, respectively, and R_2 (m) is the radius of outer barrel. For
 235 the static yields stress, it can be directly obtained by applying static torque in Eq. (8).

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Fig. 5 Rheological testing programs

240 Mini-slump test is a classic method to measure materials' workability , which is based on the
 241 spread diameter of slurry placed into a cone-shape mold [37]. The mini slump cone typically has
 242 a height of 70 mm, an internal diameter of 50 mm and 100 mm at the top and bottom,
 243 respectively. The cone is placed on a flat, level, and non-absorbent hard surface. During the test,
 244 it is filled with fresh material, and then raised after certain amount of time to allow the material
 245 to spread. After spreading ceases, two diameters of material mass are measured in orthorginal

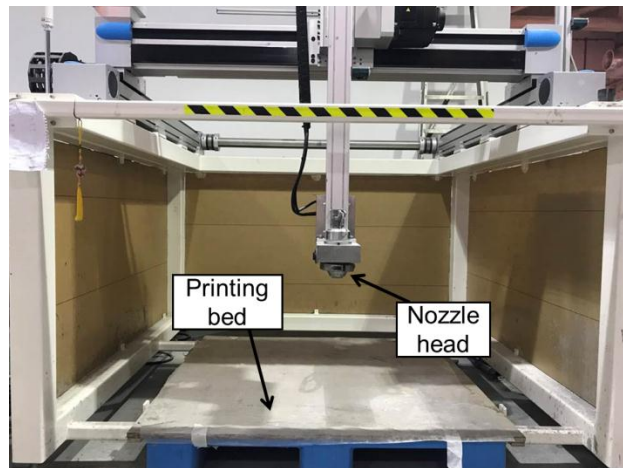
246 direction, and the workability of cementitious material can be characterized by the average of the
247 two diameters.

248

249 4.3 Printing investigation

250 Finally, printing tests were conducted to investigate the buildability of different mixtures. As
251 shown in Fig. 6, a gantry printer with a 1.2 m × 1.2 m × 1.0 m (L×W×H) printing volume was
252 used to control nozzle position for printing. As shown in Fig. 7, the materials were pumped by a
253 MAI Pictor pump from black funnel to the nozzle head through a hose with 3 m in length and
254 2.54 cm in diameter. Fig. 8 illustrates the 3D model used in the printing test. The circular column
255 with 10 cm inner diameter is composed of 50 layers. Each layer is 20 mm in width and 10 mm in
256 height theoretically.

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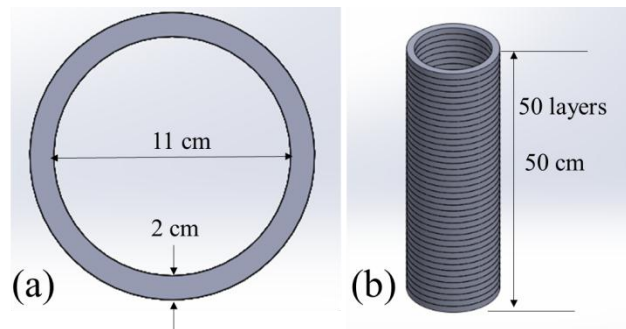
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Fig. 6 Gantry concrete -printer used for printing test



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Fig. 7 MAI Pictor pump to convey materials



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Fig. 8 3D model for printing test

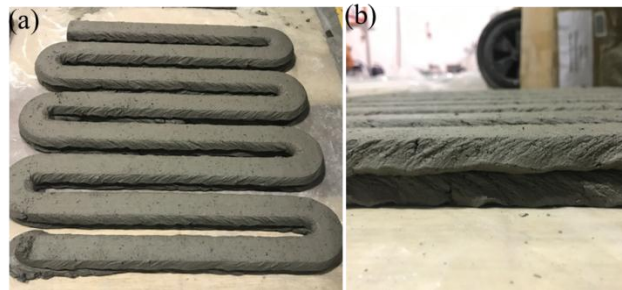
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268 4.4 Density, compressive strength and flexural performance

269 All six mixtures were casted into 50 mm × 50 mm × 50 mm cubic molds, consolidated via
270 vibration table and trowel finished. All specimens were then demolded after 24 h. All samples
271 were covered by a plastic sheet and then stored into a plastic container for curing. The density
272 was tested at 7 days, 14 days and 28 days, according to ASTM C642 [38].

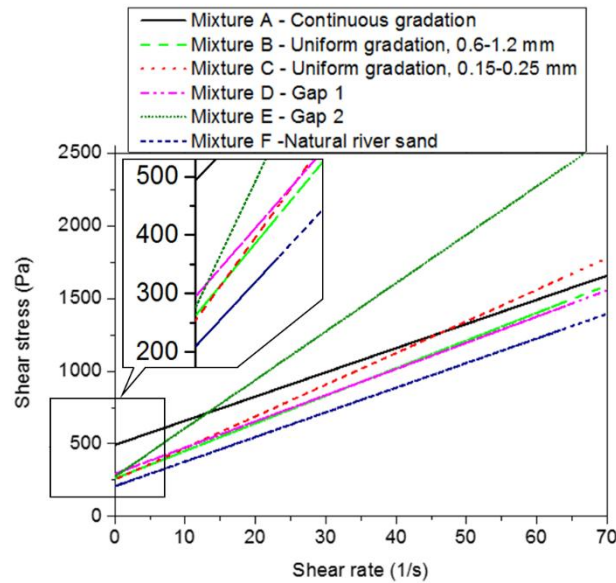
273 After density test, the compressive strength test was carried on the cubic specimens. The
274 compressive strength was measured by uniaxial loading in triplicates at 7, 14 and 28 days in

275 accordance with the specifications of ASTM C109/C109M-13 [39]. The equipment used for this
276 purpose was a Toni Technik Baustoffprüfsysteme machine with a loading rate of 100 KN/min.
277 The gantry concrete printer was used to fabricate flexural specimens. The nozzle used for
278 printing was 30 mm × 15 mm (L × W). The printing speed and pumping speed were 4,000
279 mm/min and 1.8 L/min respectively. The standoff distance was 15 mm for each layer. Printed
280 path and side view of printed filaments are shown in Fig. 9. Then, printed specimen was cut into
281 separate filaments with 350 mm in length and 300 mm in height (two layers). Afterwards, a four-
282 point bending test with a span length of 240 mm was conducted at 7 days, 14 days and 28 days.



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Fig. 9 Flexural specimen preparation: (a) top view of the printed filament; (b) side view of printed filament



289 Fig. 10 illustrates the shear stress and shear rate relation from rheological results based on
 290 Bingham Plastic model. The raw data of static torque, flow resistance and torque viscosity were
 291 shown in Table 3, which were then converted to static/dynamic yield stress and plastic viscosity
 292 via Eq.(8) and shown in the same table.
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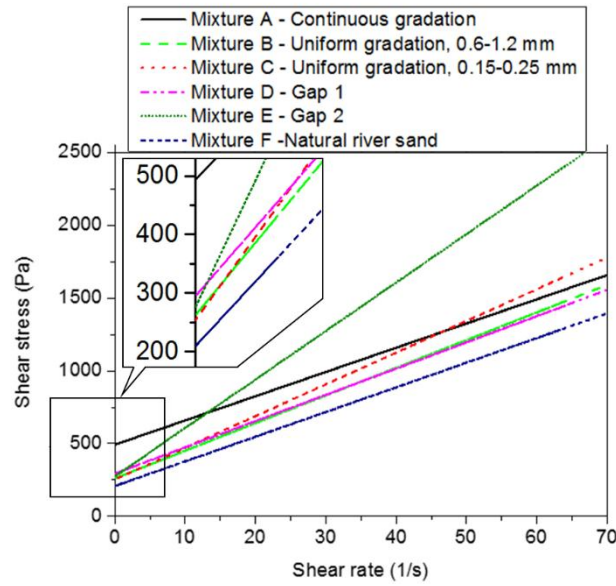


Fig. 10 Shear stress and shear rate relation based on Bingham model

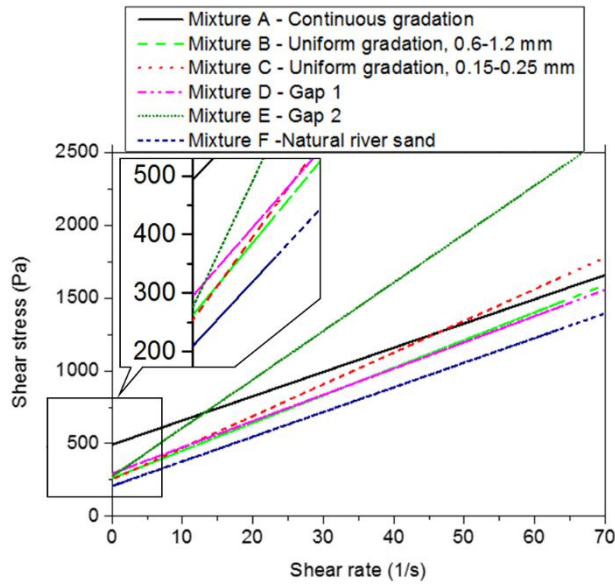
Table 3 rheological performance of different mixtures

	Static torque/ N·mm	Flow resistance/ N·mm	Torque viscosity/ (N mm/rpm)	Static yield stress/Pa	Dynamic yield stress/ Pa	Plastic viscosity/ Pa s
Mixture A	3390	498.5	2.280	3350	492.7	16.65
Mixture B	2440	264.5	2.601	2411	261.3	19.00
Mixture C	2132	255.8	2.984	2107	252.8	21.81
Mixture D	3358	287.3	2.941	3318	290.6	18.03
Mixture E	2725	277.4	4.560	2693	274.1	33.31
Mixture F	1897	210.9	2.320	1874	208.4	16.95

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300 *As can be seen from*
 301 Fig. 10 and Table 3, mixture A has the highest static yield stress and the smallest plastic
 302 viscosity, which is very desirable to ensure low pumping pressure and high buildability based on
 303 previous discussion; mixture E possesses a highest viscosity among all the mixtures, which
 304 suggests that the pumpability of mixture E could be the worst.

305 Fig. 11 reveals the results from mini-slump test. As can be seen from Fig. 11, for all mixtures,
 306 the diameter tends to decline along the time. At each time point, mixture A has the lowest slump.
 307 After 20 minutes, the spread diameter of mixture A is approximately 100 mm.

308 Rheological and mini-slump test results indicate that the most proper materials for 3DCMP is
 309 mixture A with continuous gradation. With the highest yield stresses and the lowest plastic
 310 viscosity, mixture A is expected to reveal better pumpability and buildability among all mixtures
 311 investigated in this study.

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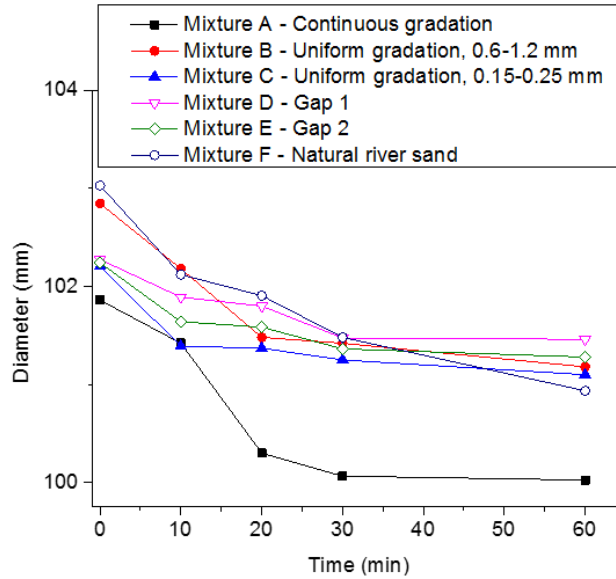


Fig. 11 Mini-slump test result

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316 5.2 Printing Test

317 A gantry printer system was used to conduct the printing test of mixtures A to F. Moving speed
318 of nozzle and pumping speed of were 9,000 mm/min and 1.8 L/min, respectively. During the
319 printing test, the printed part can maintain its shape initially. When the printed structure meets
320 the failure criteria at the fresh state, which was described by Perrot et al. [26], suddenly
321 noticeable deformation occurs. This was followed by misalignment of printing path and final
322 collapse of the printed structure. The layer numbers with noticeable deformation and final
323 collapse were recorded during the printing test.

324

325 Printing test results are summarized in Table 4, where layer number associated with noticeable
326 deformation and final collapse are presented. Taking mixture A as an example, it maintained its
327 shape until the 42nd layer (Fig. 12 (a)), followed by large deformation (Figure 12(b)) and finally
328 fell down at the 43rd layer (Fig. 12 (c)). For other mixtures, similar phenomena are observed
329 during the printing test, except that they all showed large deformation and collapsed at much

330 early stage (smaller layer number). As stated by Perrot et al. [17], the maximum build-up height
 331 can be computed by solving equation Eq. (1). For hollow cylinder, the geometric factor can be
 332 computed by

$$\begin{aligned}
 \alpha = & (R_2^2 - R_1^2)^{-1} \left(\frac{1}{2} + \frac{C_\alpha}{R_2^2} \right)^{-1} \sqrt{\frac{3}{4} + \frac{C_\alpha^2}{R_2^4}} \\
 & \cdot \left\{ \frac{4}{H} \left[\frac{(R_2^3 - R_1^3)}{6} + C_\alpha (R_2 - R_1) \right] \right. \\
 & + 2C_\alpha \left(\sqrt{\frac{3R_2^4}{4C_\alpha^2} + 1} - \sqrt{\frac{3R_1^4}{4C_\alpha^2} + 1} \right) \\
 & - 2C_\alpha \left[\operatorname{arcsinh} \left(\frac{2C_\alpha}{\sqrt{3}R_2^2} \right) - \operatorname{arcsinh} \left(\frac{2C_\alpha}{\sqrt{3}R_1^2} \right) \right] \\
 & - 2R_1^2 \left(\frac{3}{4} + \frac{C_\alpha^2}{R_1^4} \right)^{-1/2} \left(\frac{1}{4} - \frac{C_\alpha^2}{R_1^4} \right) \\
 & \left. - 2R_2^2 \left(\frac{3}{4} + \frac{C_\alpha^2}{R_2^4} \right)^{-1/2} \left(\frac{1}{4} - \frac{C_\alpha^2}{R_2^4} \right) \right\}
 \end{aligned} \tag{9}$$

334 where C_α can be solved from equation

$$\frac{\frac{1}{2} - \frac{C_\alpha}{R_2^2}}{\sqrt{\frac{3}{4} + \frac{C_\alpha^2}{R_2^4}}} - \frac{\frac{1}{2} - \frac{C_\alpha}{R_1^2}}{\sqrt{\frac{3}{4} + \frac{C_\alpha^2}{R_1^4}}} + \operatorname{arcsinh} \left(\frac{2C_\alpha}{\sqrt{3}R_2^2} \right) - \operatorname{arcsinh} \left(\frac{2C_\alpha}{\sqrt{3}R_1^2} \right) = 0 \tag{10}$$

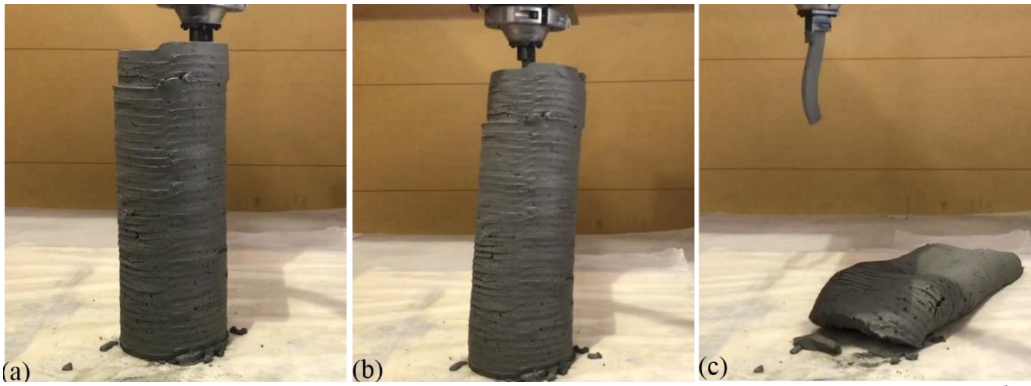
336 The detailed derivation process following geometric factor for cylinder derived by Roussel and
 337 Lanos [40] is enclosed in Appendix A. The comparison between prediction and experimental
 338 maximum built-up heights is shown in Fig. 13. As can be seen from Fig. 13, the model can well
 339 predict the deformation/collapse layer number. It is worth noting that the derivation of predicted
 340 and actual result of mixture D is quite big. As the plastic viscosity of D is high, it reduces the

341 printability and causes the discontinuity during the printing test, which reduces the stability of
 342 printed structure.

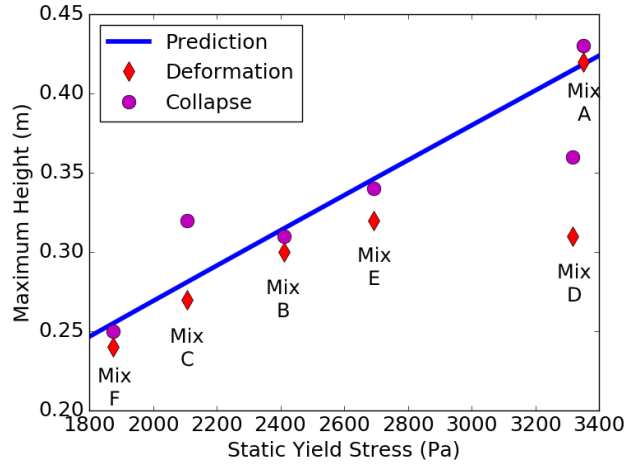
343 *Table 4 Summary of the layer number during which noticeable deformation and final collapse*
 344 *occurred*

	layer number with noticeable deformation	layer number with final collapse
Mixture A	42 nd	43 rd
Mixture B	30 th	31 st
Mixture C	27 th	32 nd
Mixture D	31 st	36 th
Mixture E	32 nd	34 th
Mixture F	24 th	25 th

345
 346
 347



348
 349 *Fig. 12 The printing test result of mixture A: (a) maintained the shape untile 42nd layer; (b)*
 350 *sudden deformation at the 42nd layer and (c) fell down at 43rd layer*
 351



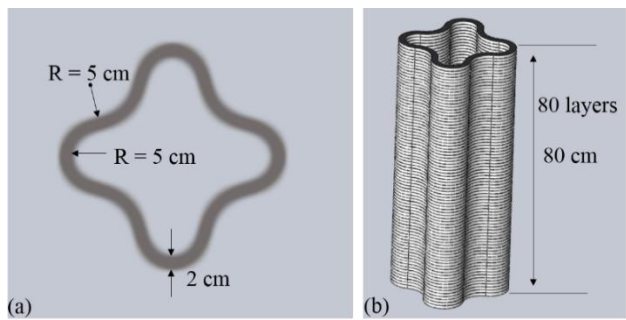
352

353 *Fig. 13. Comparison between prediction and experimental maximum built-up heights*

354

355 Afterwards, a large printing test was conducted using mixture A. A circular nozzle with 20 mm
 356 in diameter was used in this printing. Moving speed of nozzle and pumping speed of pump were
 357 9,000 mm/min and 1.8 L/min, respectively. 3D printing model is shown in Fig. 14. As can be
 358 seen from the top-view of the 3D model (Fig. 14 a), it is composed of 4 semicircles and 4 quarter
 359 circles with 10 cm in diameter and 2 cm in thickness. Then the total layer numbers are 80 for
 360 printing and the layer height is 10 mm (Fig. 14 b). The final printed part is shown in Fig. 15.

361

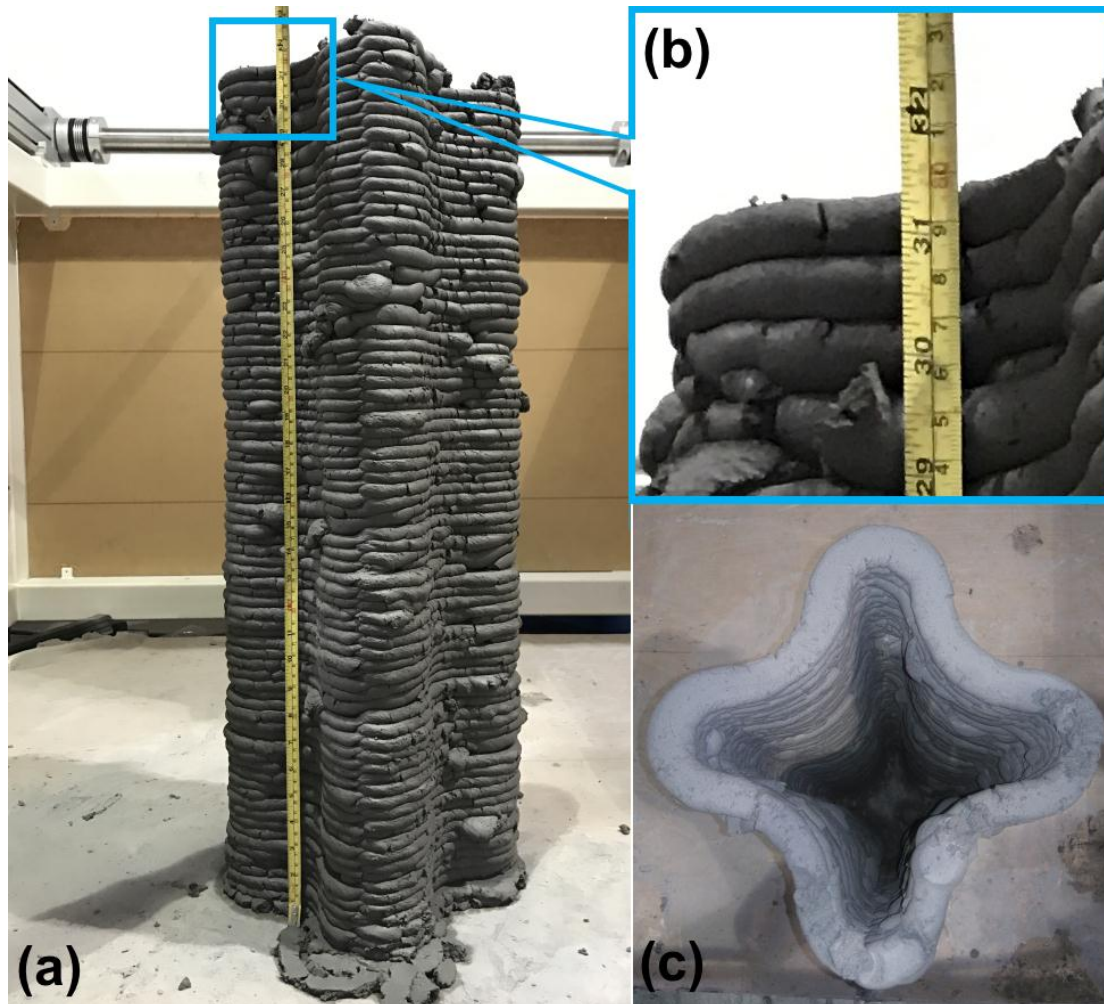


362

363

364

Fig. 14 3D model for large-scale printing: (a) top-view; (b) side-view



365
 366 *Fig. 15 Gantry printed part: (a)final printed part; (b) height and layer thickness and (c) top view*
 367 *of the printed part*
 368

369 5.3 Density, compressive strength and flexural strength

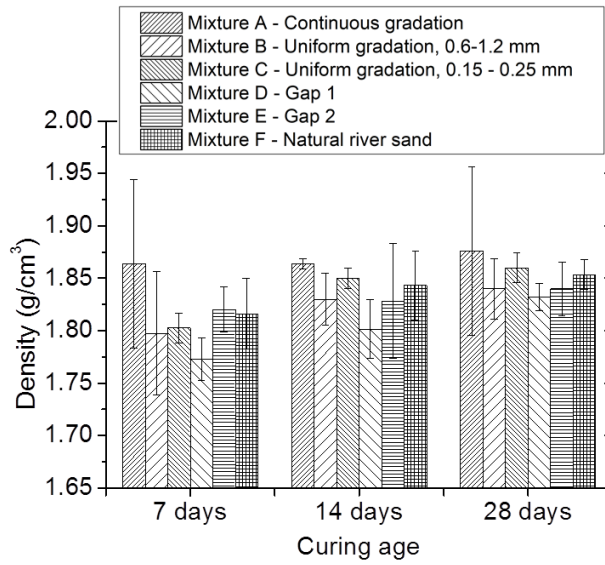
370 The density of cementitious material is related to its packing. Better packing from continuous
 371 gradation yields high density. The result of density of all mixtures is shown in Fig. 16. As
 372 expected, mixture A possesses the highest density among all mixtures at 7 days, 14 days and 28
 373 days. Furthermore, the density of all mixtures also increases with time.

374 Fig. 17 and 18 present the result of the compressive strength and flexural strength of all mixtures.

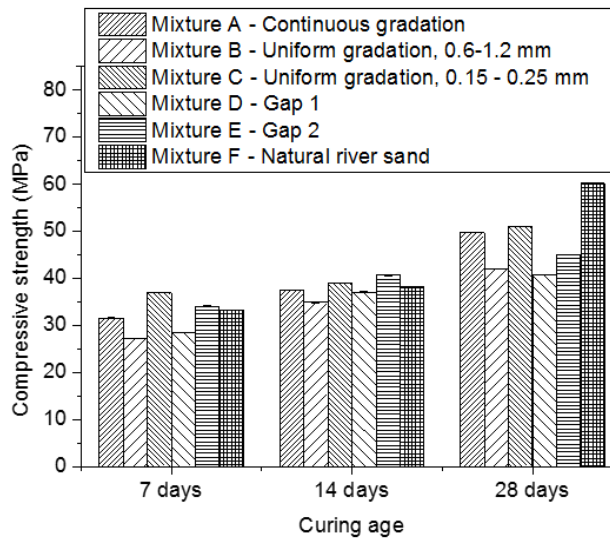
375 Mechanical strength of all mixtures increases with time as expected. Mixture A possesses

376 appropriate compressive and flexural strength at 28 days, which are 49.7 MPa and 3.73 MPa
 377 respectively.

378



379
 380 *Fig. 16 Density of all mixtures*
 381



382
 383 *Fig. 17 Compressive strength of all mixtures*
 384

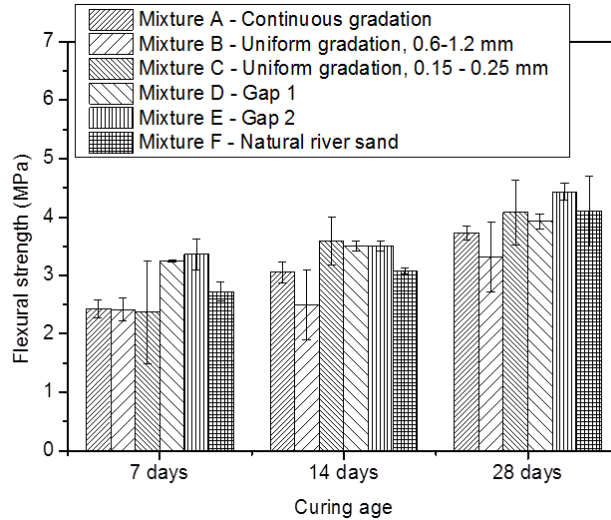


Fig. 18 Flexural strength of different mixtures

385
386
387

388 6 Summary and Conclusion

389 3DCMP requires the material to have low plastic viscosity and high yield stresses to meet the
 390 requirements for both pumpability and buildability. Fuller Thompson theory and Marson-Percy
 391 model were adopted as a guideline to design construction materials with proper rheological
 392 properties for 3DCMP. Six different mixtures (i.e., mixtures A-F) were prepared with various
 393 gradation using five different sands (0.6-1.2 mm, 0.25-0.6 mm, 0.15-0.25 mm, less than 0.15 mm
 394 and natural river sand). The gradation of mixture A followed the Fuller Thompson theory to
 395 achieve a continuous gradation system. Mixtures B and C were two different uniform-gradation
 396 mixtures with sand particles of 0.6-1.2 mm and 0.15-0.25 mm respectively. Mixtures D and E
 397 followed the gap-gradation method, where ranges of 0.25-0.6 mm and 0.15-0.25 mm were
 398 removed respectively. Mixture F used river sand with particle size bigger than 1.2 mm sieved out.
 399 Rheological tests illustrate that mixture A has the highest yield stresses and the lowest plastic
 400 viscosity. During the printing test for buildability, it can reach as high as 40 cm (40 layers)
 401 without notable deformation. On the contrary, all other mixtures appeared noticeable

402 deformation at around 30th layer. Finally, a large-scale printing with mixture A was conducted
403 and the final printed part can be as high as 80 cm without obvious deformation. Furthermore,
404 when mixture E with highest plastic viscosity was used for printing test, pumping pressure
405 significantly increased compare with others. Mechanical properties, including density,
406 compressive and flexural strength of all mixtures have also been measured.

407 In summary, Fuller Thompson method and Marson-Percy model can be used as the design
408 guideline to tailor the materials to acquire appropriate rheological properties and mechanical
409 performance for 3DCMP. More specifically, with this design guideline, desirable combination of
410 plastic viscosity and yield stress can be achieved where low plastic viscosity is essential for good
411 pumpability and high yield stresses for buildability. While the direct adoption of sand gradation
412 and mix proportion in practice is not very practical due to availability of materials and other
413 constraints, simple design guideline by adopting classic theory like Fuller Thompson method and
414 Marson-Percy model still offers meaning insights in the further development of 3D printable
415 material for building and construction industry.

416 In this research, only the sand particle gradation was investigated. In the future, it is essential to
417 extend the gradation analysis to include other filler-like materials such as silica fume and fly ash
418 in the fresh state. These materials contribute very little to the hydration process and therefore
419 mainly play the role of filler like sand at least in the fresh state. Furthermore, a more quantitative
420 link between particle size distribution/particle interactions and rheological properties needs to be
421 established in the context of 3D printing.

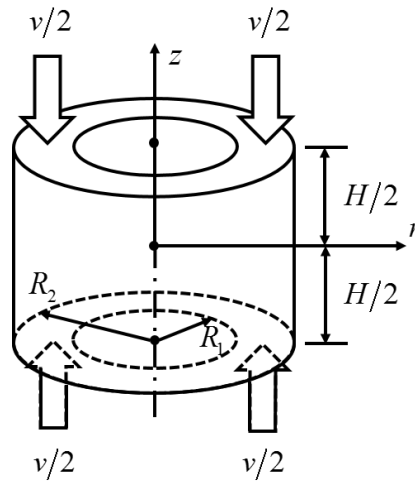
422 **Acknowledge**

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426 project

427 **Appendix A**

428 Considering a hollow cylinder has inner radius R_1 (m), outer radius R_2 (m) and height H (m),
429 which is compressed by two plates with uniform velocity v (m/s), the coordinate can be built as
430 shown in Fig. A. 1.

431



432

433 *Fig. A. 1 Schematic of a hollow cylinder compressed by constant velocity.*

434

435 The continuity equation under cylindrical coordinates without angular velocity can be written as

436
$$\frac{1}{r} \frac{\partial}{\partial r} (ru_r) + \frac{\partial u_z}{\partial z} = 0 \quad (.11)$$

437 where the velocities profiles on radial and axial directions satisfying governing equation and

438 boundary conditions are

439
$$u_r(r, z) = \frac{v}{2H}r + \frac{C}{r} \quad (.12)$$

440
$$u_z(r, z) = -\frac{v}{H}z \quad (.13)$$

441 where C is an undetermined constant. Thus, the strain rate tensor of this flow is

442
$$D_{ij} = \begin{bmatrix} \frac{v}{2H} - \frac{C}{r^2} & 0 & 0 \\ 0 & \frac{v}{2H} + \frac{C}{r^2} & 0 \\ 0 & 0 & -\frac{v}{H} \end{bmatrix} \quad (.14)$$

443 the second invariant is

444
$$I_2(r, z) = \frac{3v^2}{4H^2} + \frac{C^2}{r^4} \quad (.15)$$

445 and the stress tensor is

446
$$\begin{aligned} \sigma_{ij}^{(d)} &= \frac{K_i}{\sqrt{I_2}} D_{ij} \\ &= K_i \left(\frac{3v^2}{4H^2} + \frac{C^2}{r^4} \right)^{-\frac{1}{2}} \begin{bmatrix} \frac{v}{2H} - \frac{C}{r^2} & 0 & 0 \\ 0 & \frac{v}{2H} + \frac{C}{r^2} & 0 \\ 0 & 0 & -\frac{v}{H} \end{bmatrix} \end{aligned} \quad (.16)$$

447 In the absence of body forces and inertial stresses, the pressure governing equation is

448
$$\frac{\partial p}{\partial r} = \frac{1}{r} \frac{\partial [r\sigma_r^{(d)}]}{\partial r} - \frac{\sigma_\theta^{(d)}}{r} + \frac{\partial \sigma_{rz}}{\partial z} \quad (.17)$$

449 Integrating Eq. (.17) on $[R_1, R_2]$

450

$$\int_{R_1}^{R_2} \frac{\partial p}{\partial r} dr = \left(\frac{\nu}{2H} - \frac{C}{r^2} \right) \left[\left(\frac{3\nu^2}{4H^2} + \frac{C^2}{r^4} \right)^{-\frac{1}{2}} K_i \right]_{R_1}^{R_2} + K_i \operatorname{arcsinh} \left(\sqrt{\frac{4H^2}{3\nu^2} \frac{C}{r^2}} \right)_{R_1}^{R_2} \quad (.18)$$

= 0

451 Let $C_\alpha = HC/\nu$, Eq. (.18) can be rearranged as

452

$$\left(\frac{1}{2} - \frac{C_\alpha}{R_2^2} \right) \left(\frac{3}{4} + \frac{C_\alpha^2}{R_2^4} \right)^{-1/2} - \left(\frac{1}{2} - \frac{C_\alpha}{R_1^2} \right) \left(\frac{3}{4} + \frac{C_\alpha^2}{R_1^4} \right)^{-1/2} + \operatorname{arcsinh} \left(\frac{2C_\alpha}{\sqrt{3}R_2^2} \right) - \operatorname{arcsinh} \left(\frac{2C_\alpha}{\sqrt{3}R_1^2} \right) = 0 \quad (.19)$$

453 Apparently, 0 is the trivial solution of Eq. (.19), another solution can be solved numerically from
 454 Eq. (.19). On the plate surface, the dissipation rate is caused by the force between plates and
 455 fluids which can be computed by

456

$$2 \int_{S_p} \sigma_{rz} \left(r, \frac{H}{2} \right) u_r \left(r, \frac{H}{2} \right) dS = \frac{4\pi\nu K_i}{H} \left[\frac{(R_2^3 - R_1^3)}{6} + C_\alpha (R_2 - R_1) \right] \quad (.20)$$

457 and the dissipation rate inside of the fluid can be calculated by

$$\begin{aligned}
& \int_V \sigma_{ij}^{(d)} D_{ij} dV - \int_{S_R} \sigma_{ij}^{(d)} D_{ij} dS \\
&= 2\pi\nu K_i C_\alpha \left(\sqrt{\frac{3R_2^4}{4C_\alpha^2} + 1} - \sqrt{\frac{3R_1^4}{4C_\alpha^2} + 1} \right) \\
& - 2\pi\nu K_i C_\alpha \left[\operatorname{arcsinh} \left(\frac{2C_\alpha}{\sqrt{3}R_2} \right) - \operatorname{arcsinh} \left(\frac{2C_\alpha}{\sqrt{3}R_1} \right) \right] \\
& - 2\pi\nu K_i R_1^2 \left(\frac{3}{4} + \frac{C_\alpha^2}{R_1^4} \right)^{-1/2} \left(\frac{1}{4} - \frac{C_\alpha^2}{R_1^4} \right) \\
& - 2\pi\nu K_i R_2^2 \left(\frac{3}{4} + \frac{C_\alpha^2}{R_2^4} \right)^{-1/2} \left(\frac{1}{4} - \frac{C_\alpha^2}{R_2^4} \right)
\end{aligned} \tag{.21}$$

459 Therefore, the work applied on fluid is

$$\begin{aligned}
Fv &= \frac{4\pi\nu K_i}{H} \left[\frac{(R_2^3 - R_1^3)}{6} + C_\alpha (R_2 - R_1) \right] \\
& + 2\pi\nu K_i C_\alpha \left(\sqrt{\frac{3R_2^4}{4C_\alpha^2} + 1} - \sqrt{\frac{3R_1^4}{4C_\alpha^2} + 1} \right) \\
& - 2\pi\nu K_i C_\alpha \left[\operatorname{arcsinh} \left(\frac{2C_\alpha}{\sqrt{3}R_2} \right) - \operatorname{arcsinh} \left(\frac{2C_\alpha}{\sqrt{3}R_1} \right) \right] \\
& - 2\pi\nu K_i R_1^2 \left(\frac{3}{4} + \frac{C_\alpha^2}{R_1^4} \right)^{-1/2} \left(\frac{1}{4} - \frac{C_\alpha^2}{R_1^4} \right) \\
& - 2\pi\nu K_i R_2^2 \left(\frac{3}{4} + \frac{C_\alpha^2}{R_2^4} \right)^{-1/2} \left(\frac{1}{4} - \frac{C_\alpha^2}{R_2^4} \right)
\end{aligned} \tag{.22}$$

461 For a gravity driven hollow cylinder

$$462 \quad F = \rho g H \pi (R_2^2 - R_1^2) \tag{.23}$$

463 when the fluid starts to flow, it must have

$$464 \quad \tau_s \alpha = \rho g H \tag{.24}$$

465 where

466
$$\min \left[\sigma_{\theta}^{(d)} \right] = \sigma_{\theta}^{(d)} \Big|_{r=R_2} = \tau_s \quad (.25)$$

467 Substituting Eqs. (.23) to (.25) into Eq. (.22) and rearranging

468
$$\begin{aligned} \alpha = & (R_2^2 - R_1^2)^{-1} \left(\frac{1}{2} + \frac{C_{\alpha}}{R_2^2} \right)^{-1} \sqrt{\frac{3}{4} + \frac{C_{\alpha}^2}{R_2^4}} \\ & \cdot \left\{ \frac{4}{H} \left[\frac{(R_2^3 - R_1^3)}{6} + C_{\alpha} (R_2 - R_1) \right] \right. \\ & + 2C_{\alpha} \left(\sqrt{\frac{3R_2^4}{4C_{\alpha}^2} + 1} - \sqrt{\frac{3R_1^4}{4C_{\alpha}^2} + 1} \right) \\ & - 2C_{\alpha} \left[\operatorname{arcsinh} \left(\frac{2C_{\alpha}}{\sqrt{3}R_2^2} \right) - \operatorname{arcsinh} \left(\frac{2C_{\alpha}}{\sqrt{3}R_1^2} \right) \right] \\ & - 2R_1^2 \left(\frac{3}{4} + \frac{C_{\alpha}^2}{R_1^4} \right)^{-1/2} \left(\frac{1}{4} - \frac{C_{\alpha}^2}{R_1^4} \right) \\ & \left. - 2R_2^2 \left(\frac{3}{4} + \frac{C_{\alpha}^2}{R_2^4} \right)^{-1/2} \left(\frac{1}{4} - \frac{C_{\alpha}^2}{R_2^4} \right) \right\} \end{aligned} \quad (.26)$$

469 where C_{α} can be solved from Eq. (.19).

470 .

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